TRANSPORTATION CABINET DEPARTMENT OF HIGHWAYS **WOLFE-MORGAN COUNTIES** RAMP A OVER RED RIVER STA. 22 + 33.50

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BID ITEM CODE	08100	08104	08150	08151	08001	08002	08019	02231	02998	08046	08033	08039	08094	03299	21532ED	08634	21420ED	21421ED	20745ED	20746ED								BG)
BID ITEM	Concrete Class "A"	Concrete Class "AA"	Steel Reinforcement	Steel Reinforcement, Epoxy Coated	Structure Excavation, Common	Structure Excavation, Solid Rock	Cyclopean Stone Rip Rap	Structure Granular BackfIII	Masonry Coating	Piles - Steel HP 12 x 53	Test Piles	Pre-Drilling for Piles	Pile Points 12 Inch	Armored Edge for Concrete	Railing System Type 3	Precast PC Feam Type 4	Drilled Shaft 66" (Common)	Drilled Shaft 60" (Rock)	Rock Soundings	Rock Corings								BBP BBP BGX BGX RGX RGX BJE BPS
UNIT	C.Y.	C.Y.	LBS.	LBS.	C.Y.	C.Y.	Tons	C.Y.	S.Y.	L.F.	L.F.	L.F.	EA.	L.F.	L.F.	L.F.	L.F.	L.F.	L.F.	L.F.								
Integral End Bent #1	17.8			3550	81	4	552	69	27	62	28	60	Θ 6	5														
Pier #1	50.8	13.5	9436	72	36		-		50				1		-		41	16		46		+	_					
Pier #2	47.4	13.5	8543	75.03	33		453	7.	50	100	.,,	-	!		-	-	46	16	46	46	 +	+-	+	+-		-		
Integral End Bent #2	18.2	24.9		3597	-		453	"	28	166	47	-	+ •	1		-		1				+	+					
Pier *2 Integral End Bent *2	\blacksquare	 										-	1			-		-	1		-	-	-	-				
l ₀	1	 			-							-	-		+	-	-	-	+	-	 +	+	+	+-	_			
	1 1											-	1		1	-		1	1	-	+	+	+	_				
Superstructure		173.7		50047					875					58	470	922.5												
BRIDGE TOTALS	134.2	249.9	17979	57338	150	4	1005	140	1030	228	75	60	12	58	470	922.5	87	32	87	92								201

	INDEX OF SHEETS
Sheet No	Description
S1	Title Sheet
S2	General Notes
S3	Layout
S4-S7	Subsurface Data
S8	Geometric Layout
S9-S10	Foundation Layout
SII-S12	Integral End Bent 1
S13-S15	Pier Details
S16-S17	Integral End Bent 2
S18	Integral End Bent Details
S19	Framing Plan
S20	PPC I-Beam Type 4
S21-S24	Superstructure
S25-S26	Construction Elevations
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S28	Steel Diaphragms
	SPECIAL NOTES
11C Dril	ed Shafts

SPECIAL PROVISIONS

69 Embankment at Bridge End Bent Structures

STANDARD DRAWINGS

BBP-001-12	Elastomeric Bearing Pads for Prestressed Beams
BBP-002-04	Bearing Details
BGX-006-10	Stencils for Structures
BGX-012-02	Geotechnical Legend
RGX-100-06	Treatment of Embankment at End Bents
RGX-105-08	Treatment of Embankment at End Bents
BJE-001-13	Neoprene Expansion Dams and Armored Edges
BPS-003-09	HPI2x53 Steel Pile
	OPERIORIONIO

SPECIFICATIONS

2012 Standard Specifications for Road and Bridge Construction.

2012 AASHTO LRFD Bridge Design Specifications, 6th Edition.



ITEM NUMBER

10-126.70

CHECKED BY DATE: June. 2016 DESIGNED BY: B.C. REID DETAILED BY: W.R. ABBOTT B.C. REID Commonwealth of Kentucky

DEPARTMENT OF HIGHWAYS

WOLFE-MORGAN

RED RIVER TITLE SHEET

LOCHNER

RAMP A

H.W. LOCHNER, INC. LEXINGTON, KENTUCKY



GENERAL NOTES

SPECIFICATIONS: References to the Specifications are to the current edition of the Kentucky Department of Highways Standard Specification for Road and Bridge Construction including any current Supplemental Specification. All references to the AASHTO Specifications are to the sixth edition of the AASHTO LRFD Bridge Design Specifications for Highway Bridges.

DESIGN LOAD AND METHOD: This bridge is designed for KY HL-93 live load. The KY HL-93 live load is arrived at by increasing the Standard KY HL-93 truck and lane loads as specified in the AASHTO Specifications by 25%. All reinforced concrete members are designed by the load and resistance factor method as specified in the current AASHTO Specification.

WIND LOAD: This bridge is designed for a wind load based on a wind velocity of 100 mph.

FUTURE WEARING SURFACE: This bridge is designed for a 60 psf future wearing surface.

MATERIALS DESIGN SPECIFICATION:

FOR CLASS "A" REINFORCED CONCRETE F'C = 3500 PSI FOR CLASS "AA" REINFORCED" CONCRETE F'C = 4000 PSIFOR STEEL REINFORCEMENT FY = 60000 PSI FOR STEEL PILING FY = 50000 PSI

CONCRETE: Use Class "AA" concrete in the superstructure deck. parapet, and diaphragms. Class "A" Concrete is to be used in substructure. Prestressed girder concrete shall be in accordance with the plans and specifications.

REINFORCEMENT: Dimensions shown from the face of concrete to bars are to center of bars unless otherwise shown. Spacing is from center to center of bars. Clear distance to face of concrete is 2", unless otherwise noted. Epoxy coat bars desginated by suffix (e) in accordance with Section 811.10 of the Standard Specifications. Use stirrup bend diameters for bars designated by suffix (s) in a Bill of Reinforcement.

BEVELED EDGES: Bevel all exposed edges $\frac{3}{4}$, unless otherwise noted.

SHOP DRAWINGS: Submit shop drawings that are required by the plans and specifications directly to the Bridge Consultant. If any changes in the design plans are proposed by a fabricator of supplier, submit those changes to the Bridge Consultant through the Contractor. The Bridge Consultant shall provide a copy of the final approved shop plans to the Division of Structural Design.

DIMENSIONS: Dimensions are for a normal temperature of 60 degrees Fahrenheit. Layout dimensions are horizontal dimensions.

SLOPE PROTECTION: Use dry cyclopean stone in accordance with the plans and Specifications. Geotextile Fabric is to be incidental to this item.

PILE POINTS: Provide pile points for all point bearing piles. Ensure pile points are in accordance with Section 604 of the Specifications and of the type shown on the Foundation Layout Sheet.

PILING: Piling shall be driven to refusal. Test piles shall be driven where designated on the plans to determine the length of pile required. All test piles shall be accurately located so they may be used in the structure.

COMPLETION OF THE STRUCTURE: The Contractor is required to complete the structure in accordance with the plans and specifications. Material, labor or construction operations, not otherwise specified, are to be included in the bid item most appropriate to the work involved. This may include cofferdams, shoring, excavations, backfilling, removal of all or parts of existing structures, phase construction, incidental materials, labor or anything else required to complete the structure.

SPIRAL COLUMN TIES: Splices for spirals where desired by the contractor shall be made with a minimum of one and one-half turns of spiral. No additional payment will be made for these splices, and the cost will be considered incidental to the cost of the developed-length of spiral shown on the plans. Spiral reinforcement shall meet the requirements of subsection 811.02.01 of the Specifications.

The length shown in the bill of reinforcement for spirals is the distance from top of footing to bottom layer of reinforcement in the pier cap. The number of turns shown is the length divided by the pitch, plus 3 turns (total number of closed coils) expressed to the nearest whole number. One and one-half closed coils shall be provided at the ends of each spiral unit. 4 channel, tee or angle spacers, weighing approximately 0.8 lbs. per linear foot of spacer, shall be provided for each spiral unit. They shall be equally spaced along the periphery of the coil. Weight of spiral reinforcement is included in the estimate of quantities for each pier.

POURING SEQUENCE: The pouring sequence of the slab may not be changed without the written approval of the Engineer.

MASONRY COATING: Apply masonry coating to the concrete surfaces as specified in Section 601.03.18 (B).

END BENT CONSTRUCTION: Geotextile fabric and perforated pipe installed in accordance to Special Provision 69 shall be considered incidental to unit price bid for Structure Granular Backfill.

GEOTECHNICAL INFORMATION: Additional information can be found in geotechnical report S-017-2014.

CONSTRUCTION IDENTIFICATION: The names of the Prime Contractor and any sub-contractors shall be imprinted in th econcrete in accordance with Standard Drawing BGX-006, c.e. at a location designated by the Engineer. The contractor shall furnish all plans, equipment, and labor necessary to do the work for which no direct payment will be made.

The following abbreviations may have been used in the preparation of these plans: Retween bet. b.f. Back Face Bottom of Footing BOF bot. Bottom Bearing Bra. C to C Center to Center Current Edition c.e. Cubic Yard C.Y. Chord Chd. CL Center Line CI. Clear Conc. Concrete Cu. Cubic Drawina Dwa e.f. Each Face Flevation FI. Equal ea. E.S. Equally Spaced Est. Estimate Ext. Exterior Face to Fac F to F f.f. Front Face Inside Diameter IEB Integral End Bent Inch Interior Int. Low Bridge Seat LBS LBS. Pounds М Meter MPH Miles per Hour Near Side n. s. Outside Diameter O. D. Opposite Opp. Point of Curve PC Perpendicular Pern Point of Intersection PΙ Precast Prestressed Concrete PPC PPCD Precast Prestressed Concrete Deck Unit PSI Pounds per Square Inch Point of Tanaent Radius Right RCRC Reinforced Concrete Box Culvert RCDG Reinforced Concrete Deck Girder Req'd Required Railroad Shoulder Shld spa. Spaces Station Std. Standard Str. Straight Tan Tangent REVISION DATE Thru Through OATE: June, 2016 CHECKED BY TOF Top of Footing

DESIGNED BY: B.C. REID W.D. BURTON DETAILED BY: W.R. ABBOTT W.D. BURTON Commonwealth of Kentucky DEPARTMENT OF HIGHWAYS

WOLFE-MORGAN

ROUTE A RED RIVER

H.W. LOCHNER, INC.

LEXINGTON, KENTUCKY

GENERAL NOTES LOCHNER

ITEM NUMBER

Tot.

Typ.

w P

Yd.

Vert.

Total

Yard

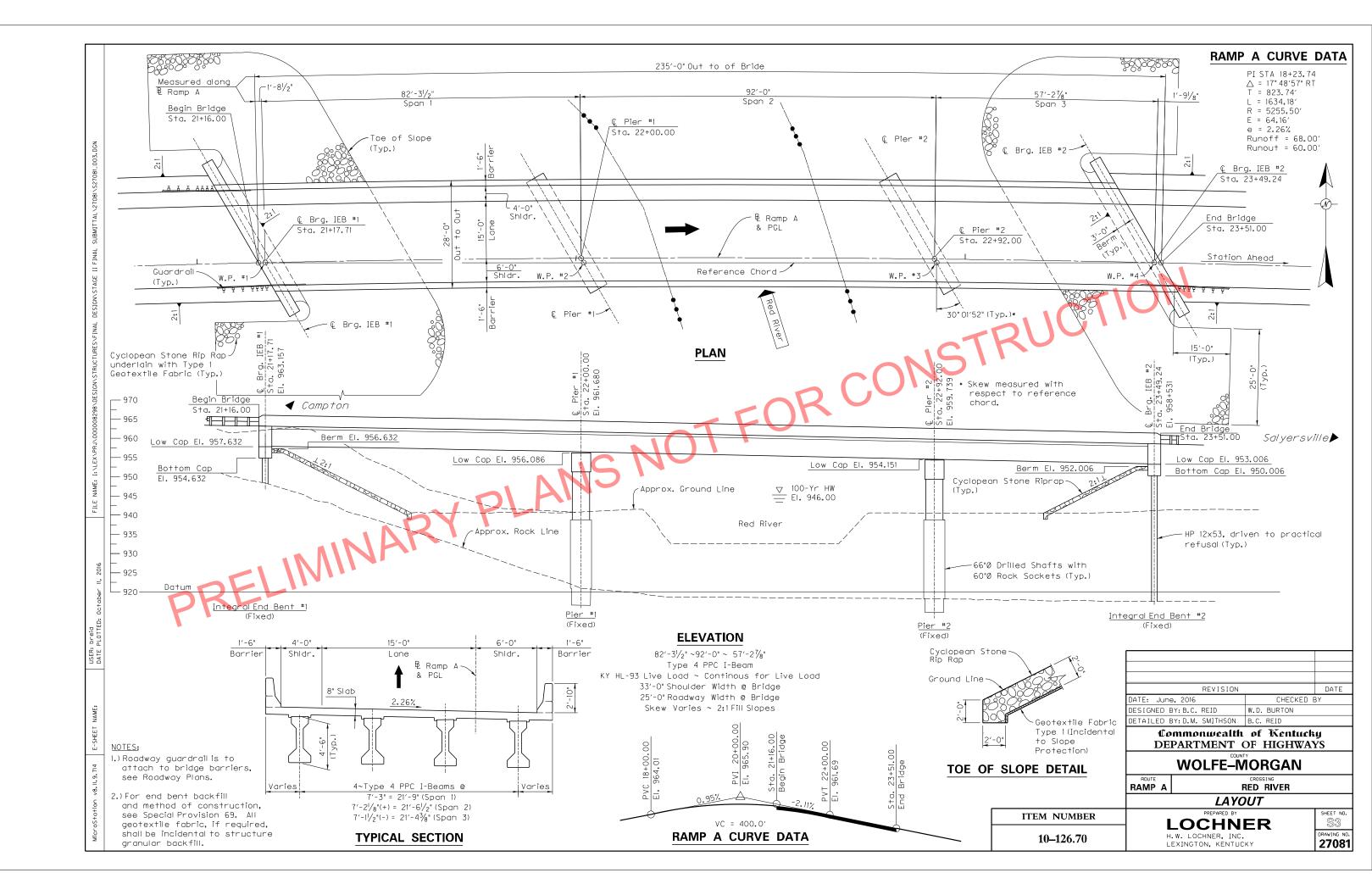
Typical

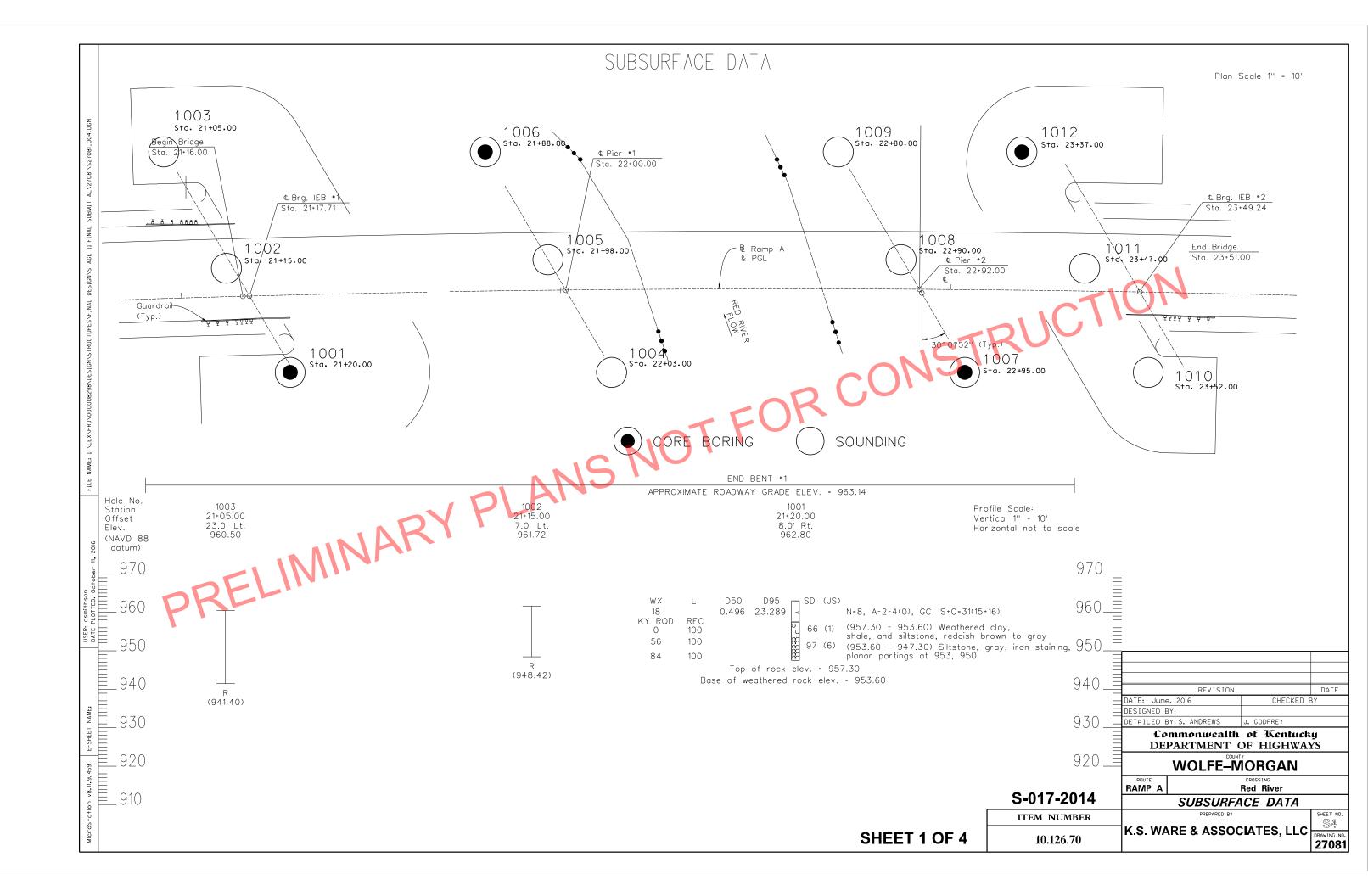
Vertical

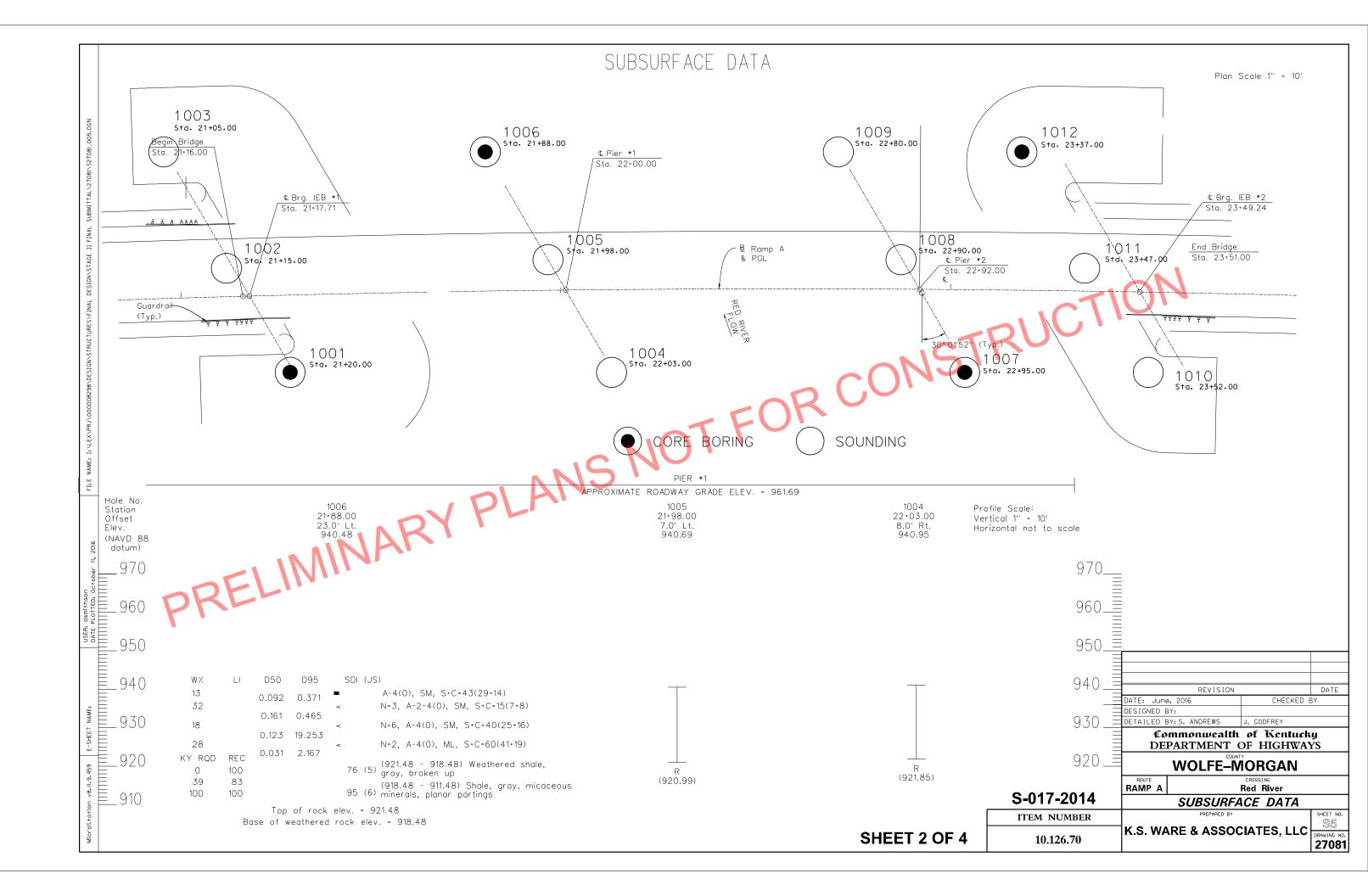
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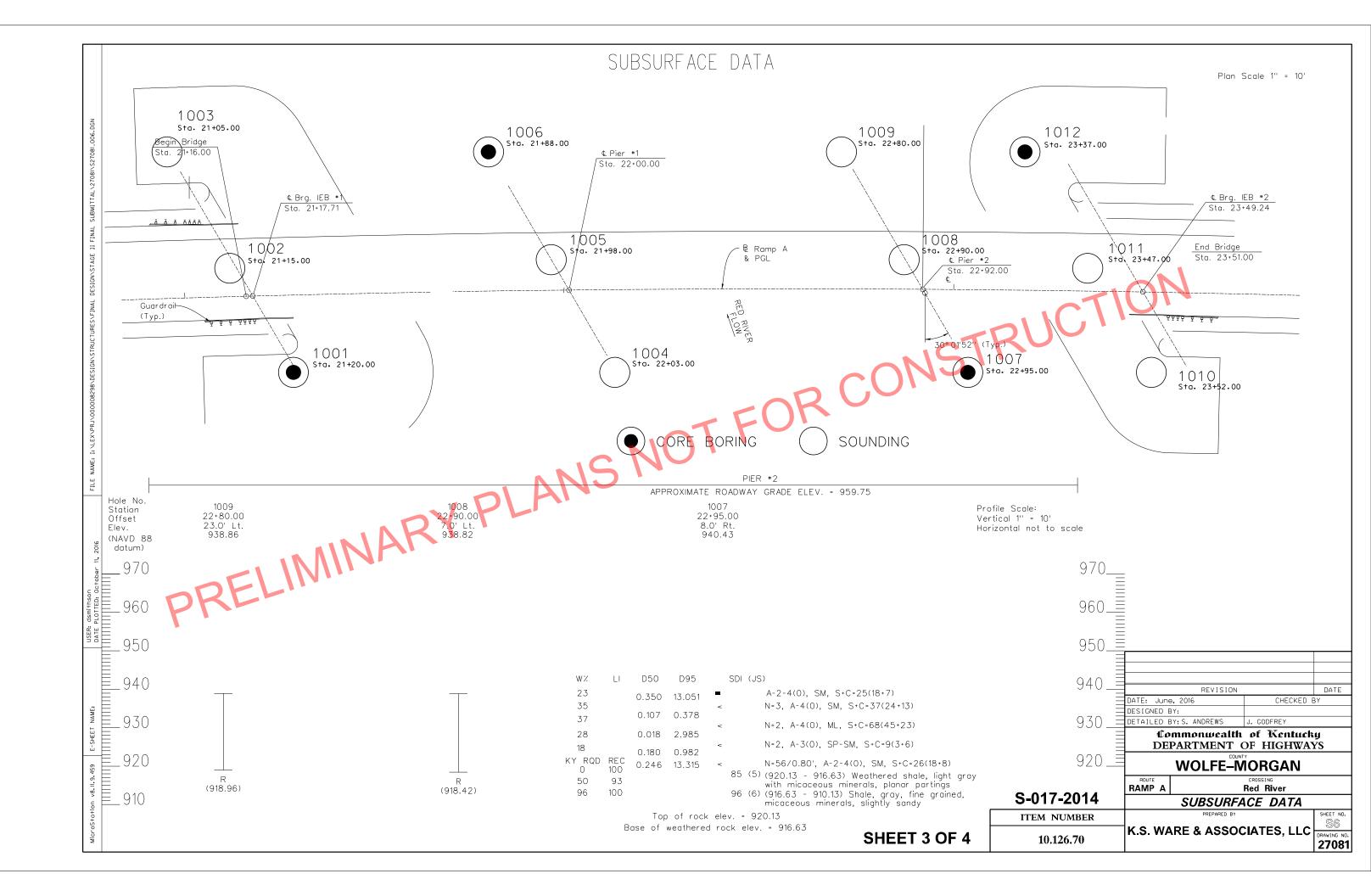
Working Point

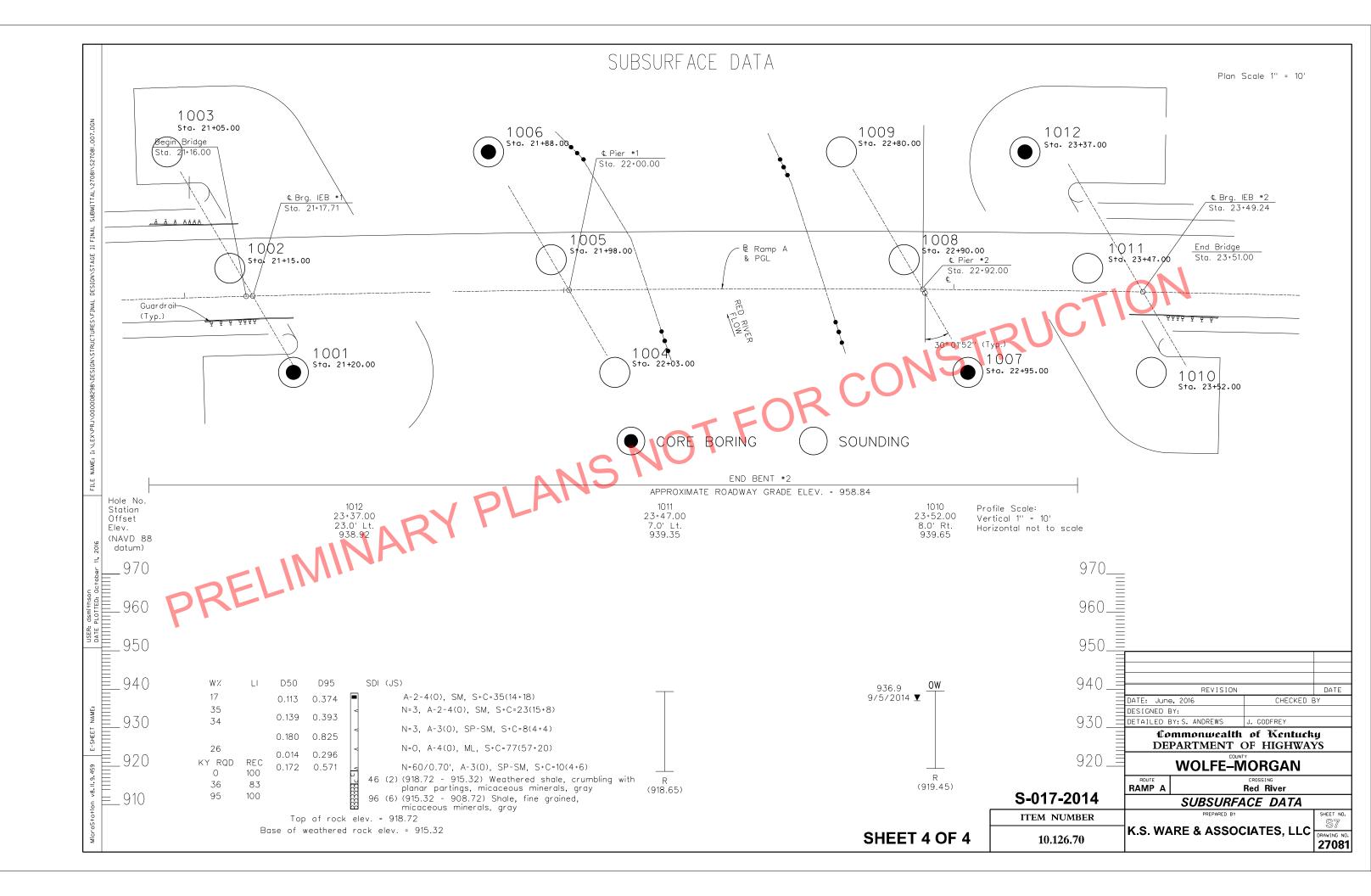


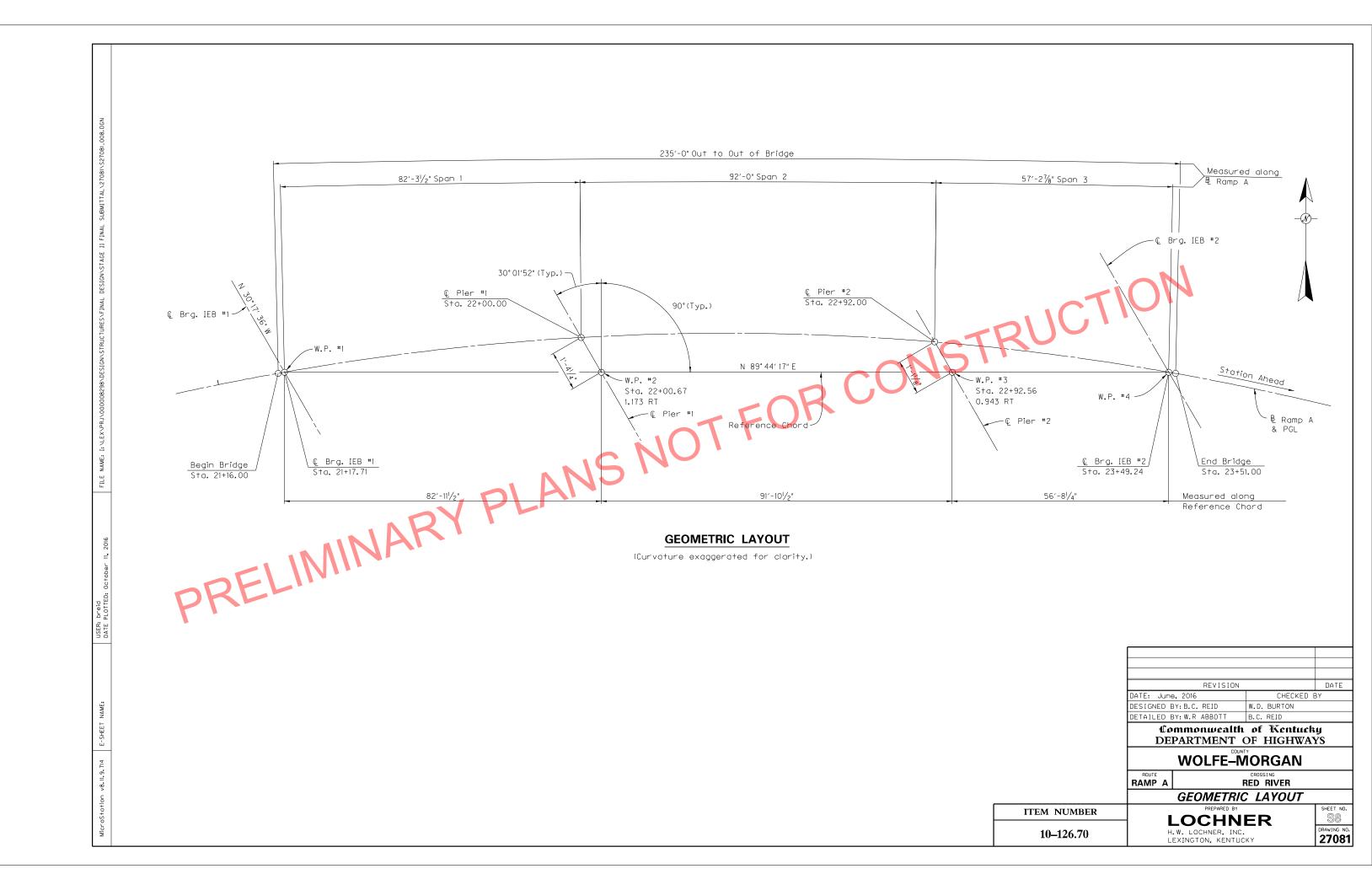


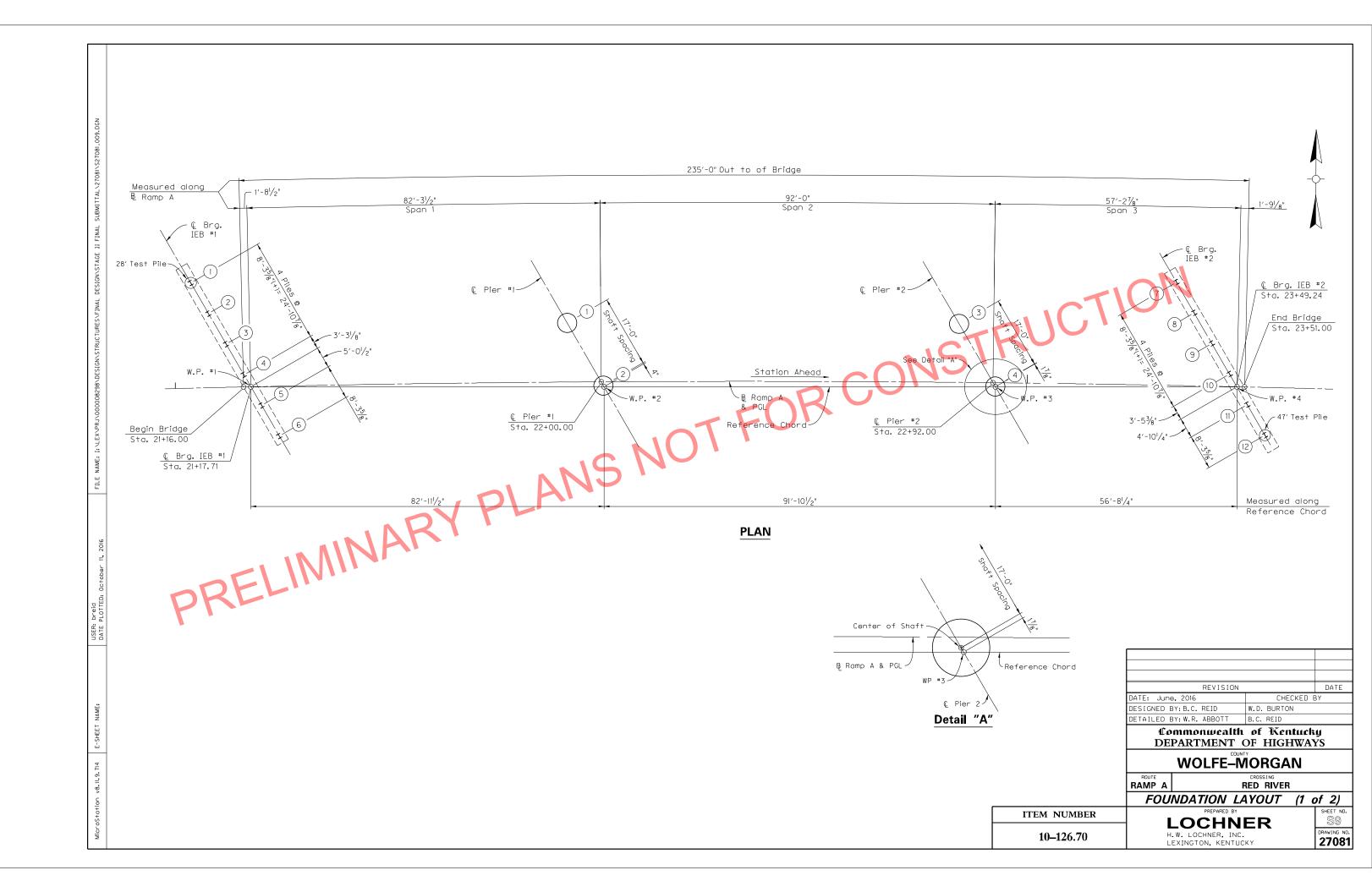












Pile Record for Point Bearing Piles – IEB #1											
Pile No.	Pile Cut-off Elevation (Feet)	Pile Length in Place (Feet)	Point of Pile Elevation as Driven (Feet)	Design Axial Load (Tons)							
1	956.632			73							
2	956.632			73							
3	956.632			73							
4	956.632			73							
5	956.632			73							
6	956.632			73							

	Pile Record for Point Bearing Piles – IEB #2											
Pile No.	Pile Cut-off Elevation (Feet)	Pile Length in Place (Feet)	Point of Pile Elevation as Driven (Feet)	Design Axial Load (Tons)								
7	952.006			73								
8	952.006			73								
9	952.006			73								
10	952.006			73								
- 11	952.006			73								
12	952.006			73								

Miscellaneous

At the south end of End Bent I, solid rock excavation may be necessary in order to reach the required base of pile cap elevation. $\,$

Slope Protection

Slope protection will be required at the bridge meeting the requirements of sections 703 and 805 of the Standard Specifications of Road and Bridge Construction, current edition. The limits, size, and thickness of the slope protection shall be as specified in HEC 23. Place a Type I Geotextile Fabric, in accordance with Sections 214 and 843 of the Standard Specifications of Road and Bridge Construction, current edition, between the embankment and the slope protection.

Predrilling For Piles

Pre-drilling will be necessary for pile installation at the south end of End Bent 1 (Station 21+20, 8 ft RT and Station 21+15, 7 ft LT). Holes will need to be drilled into solid rock in order to ensure a minimum pile embedment of 10 feet below the bottom of the cap.

The holes

shall be drilled to an elevation of 944.6 feet at End Bent 1. If the rock line is encountered below these elevations then pre-drilling shall continue until the pile bears on solid rock but drilling into bedrock will not be required. Backfill the holes with sand or pea gravel after the pile is placed in the hole. A temporary casing will be required to prevent collapse of the hole. Remove the casing as the hole is being backfilled. Drive piles to practical refusal after backfill operations are complete. Include the cost of all materials, labor, and equipment needed to pre-drill, backfill the holes, and drive the piles to refusal in the price per linear foot "Pre-drilling for Piles".

	Drilled Shaft Record													
Drill Shaft No.	Top of Drilled Shaft Common Elevation (Design)	Top of Drilled Shaft Common Elevation (Actual)	Top of Drilled Shaft Solid Rock Elevation (Design)	Top of Drilled Shaft Solid Rock Elevation (Actual)	Bottom of 66"Steel Casing Elevation (Design)	Bottom of 66"Steel Casing Elevation (Actual)	Bottom of Drilled Shaft Solid Rock Elevation (Design)	Bottom of Drilled Shaft Solid Rock Elevation (Actual)	Total Length of Drilled Shaft Common	Total Length of Drilled Shaft Solid Rock				
					PIER #I									
1	938.900		918.500		918.500		910.500							
2	938.900		918.500		918.500		910.500							
	PIER #2													
3	938.900		916.000		916.000		908.000							
4	938.900		916.000		916.000		908.000							

Hammer Criteria

Single-acting diesel hammers with rated energies between 23 kip-ft and 40 kip-ft are recommended to adequately drive the H-piles to practical refusal without encountering excessive blow counts or overstressing the piles. The use of hammers other than single-acting diesel may require different energies. The contractor shall submit the propose pile driving system to the Department for approval prior to the installation of the first pile. Approval of the pile driving system by the Engineer will be subject to satisfactory field performance of the pile driving procedures.

Drilled Shaft Foundation for Piers

See Sheets S16 and S17 for Drilled Shaft Notes.

Field Data

For each pile, the Project Engineer shall record the following on this sheet: Pile Length in Place and Point of Pile Elevation as Driven.

Submit this record to: Kentucky Transportation Cabinet
Director, Division of Structural Design
Room #322
200 Mero Street
Frankfort, KY 40622

This pile record does not replace other pile records the Project Engineer is required to keep and submit.

Use HP 12x53 in accordance with BPS-003, c.e.

Use Grade 50 steel H-Piles with reinforced pile points as end bearing piles.

Driving Criteria

DRIVING CRITERIA: Drive point bearing piles to practical refusal.

PRACTICAL REFUSAL: For this project minimum blow requirements are reached after total penetration becomes $\frac{1}{2}$ or less for 10 consecutive blows, practical refusal is obtained after the pile is struck an additional 10 blows with total penetration of $\frac{1}{2}$ or less. Advance the production piling to the driving resistances specified above and to depths determined by test pile(s) and subsurface data sheet(s). Immediately cease driving operations if the pile visibly yields or becomes damaged during driving. If hard driving is encountered because of dense strata or an obstruction, such as a boulder before the pile is advanced to the depth anticipated, the Engineer will determine if more blows than the average driving resistances specified for practical refusal is required to further advance the pile. Drive additional production and test piles if directed by the Engineer.

Definitions of Terms

PILE CUT-OFF ELEVATION: Elevation of the top of pile in the finished structure.

PILE LENGTH IN PLACE: Actual pile length below the Pile Cut-Off
Elevation in the finished structure.

POINT OF PILE ELEVATION AS DRIVEN: Actual point of pile elevation in the finished structure.

DESIGN AXIAL LOAD: Load carried by each pile as estimated from structural design calculations for Factored LRFD Loadings.

CALCULATED FIELD BEARING: Contrary to Section 604.03.07 of the Standard Specifications, in place bearing values are not required for piles bearing on rock when driven to practical refusal.

REVISION DATE

DATE: June, 2016 CHECKED BY

DESIGNED BY: B.C. REID W.D. BURTON

DETAILED BY: W.R. ABBOTT B.C. REID

Commonwealth of Kentucky DEPARTMENT OF HIGHWAYS

WOLFE-MORGAN

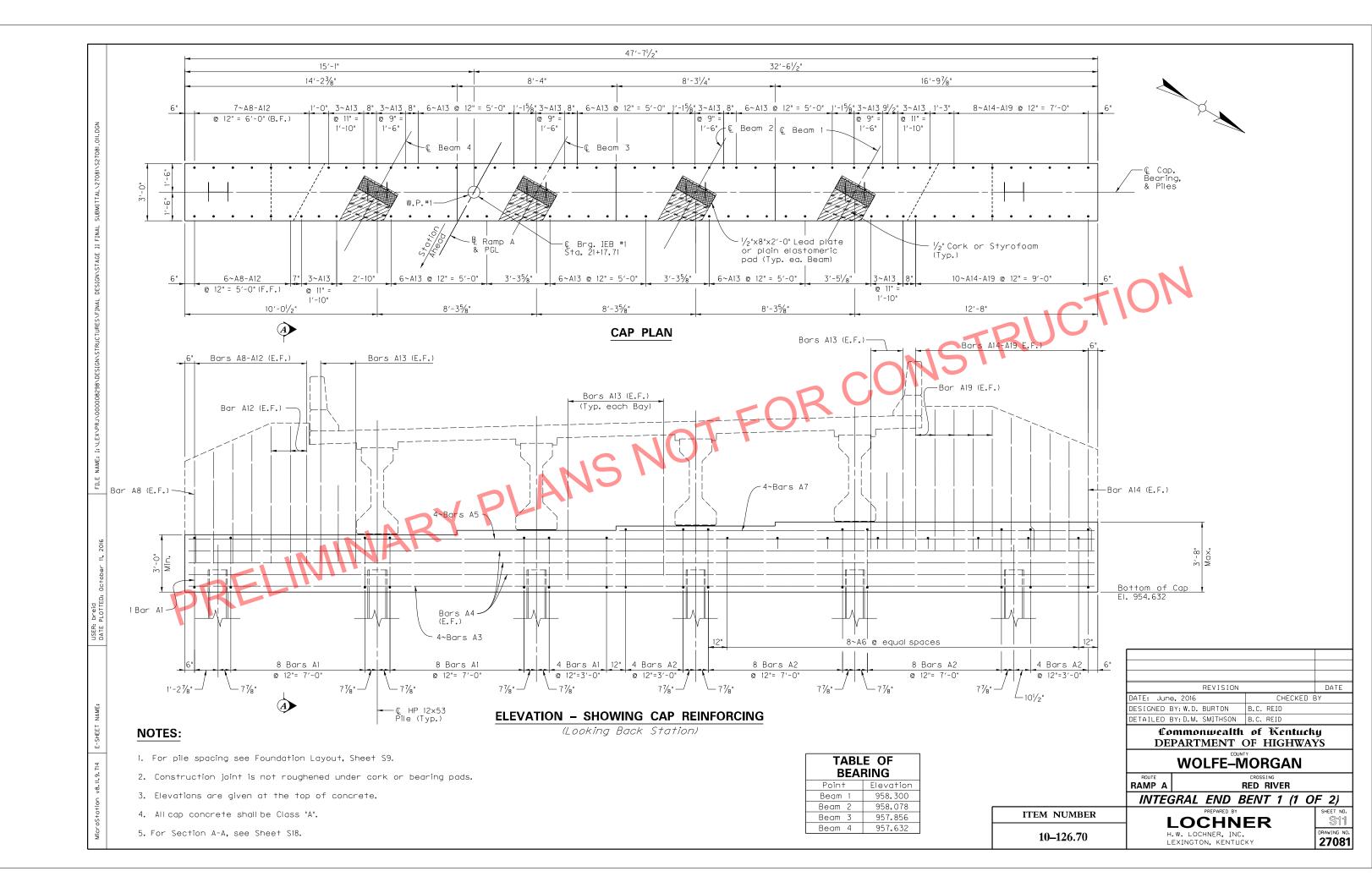
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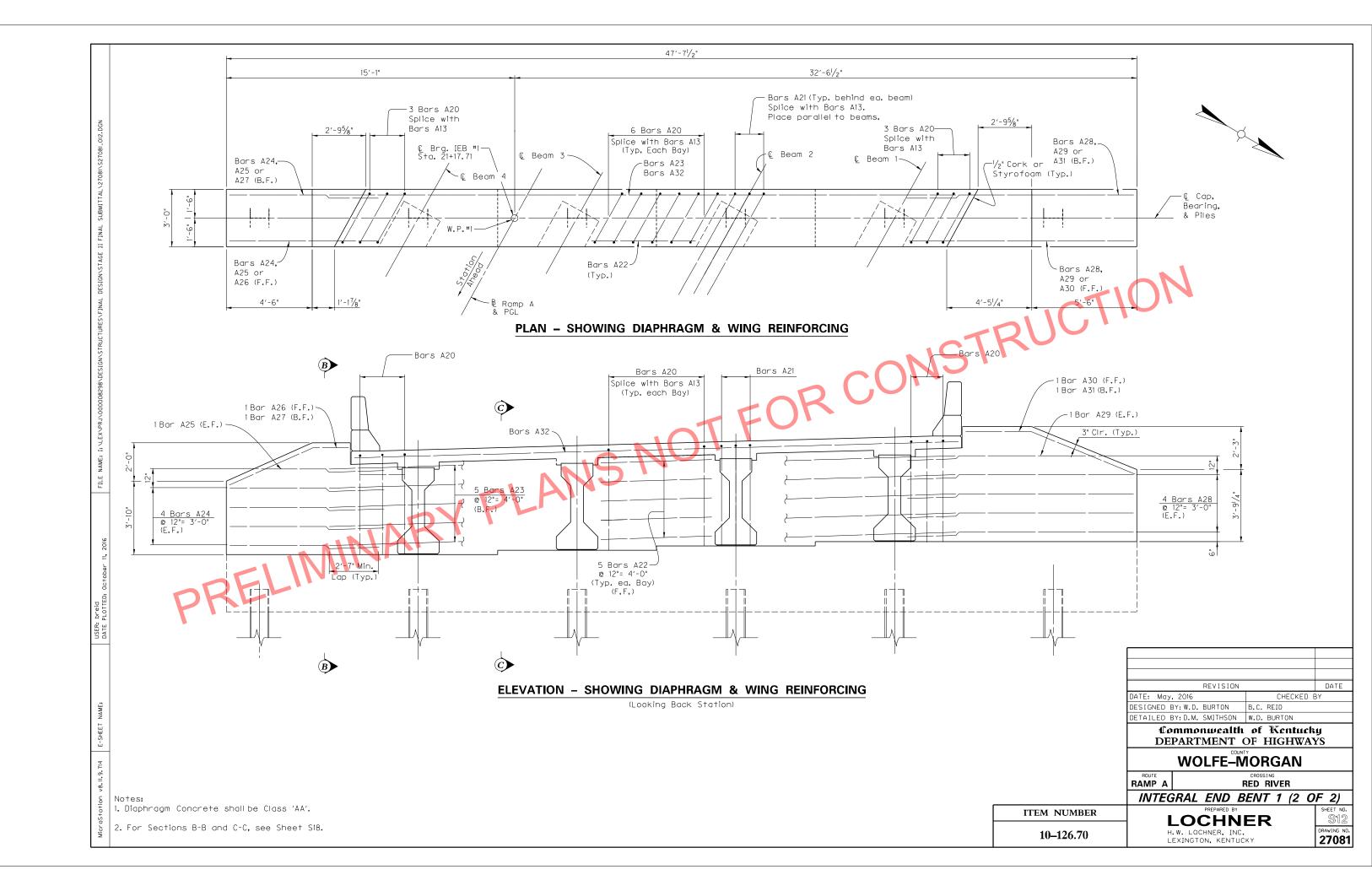
FOUNDATION LAYOUT (2 of 2)

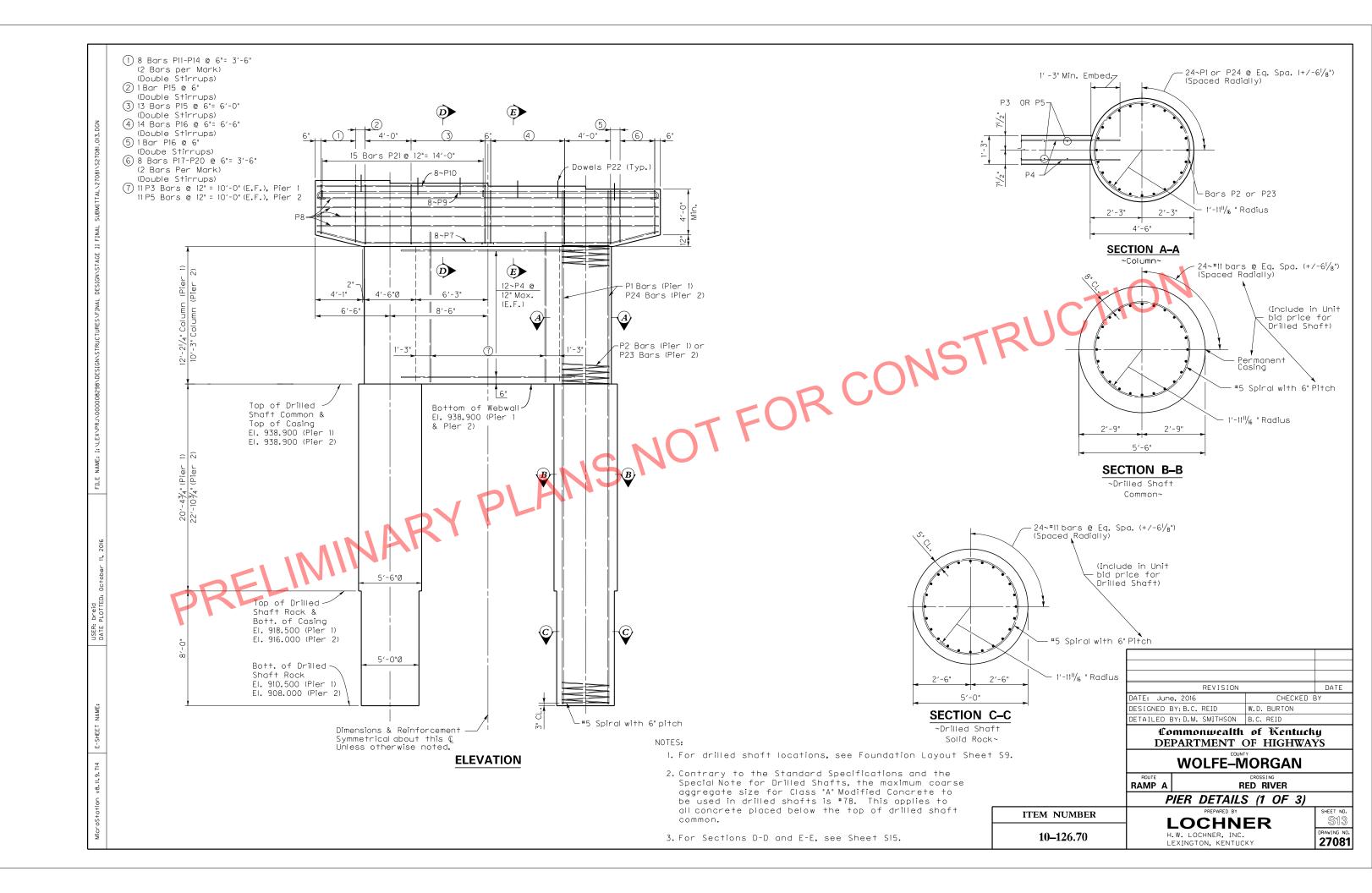
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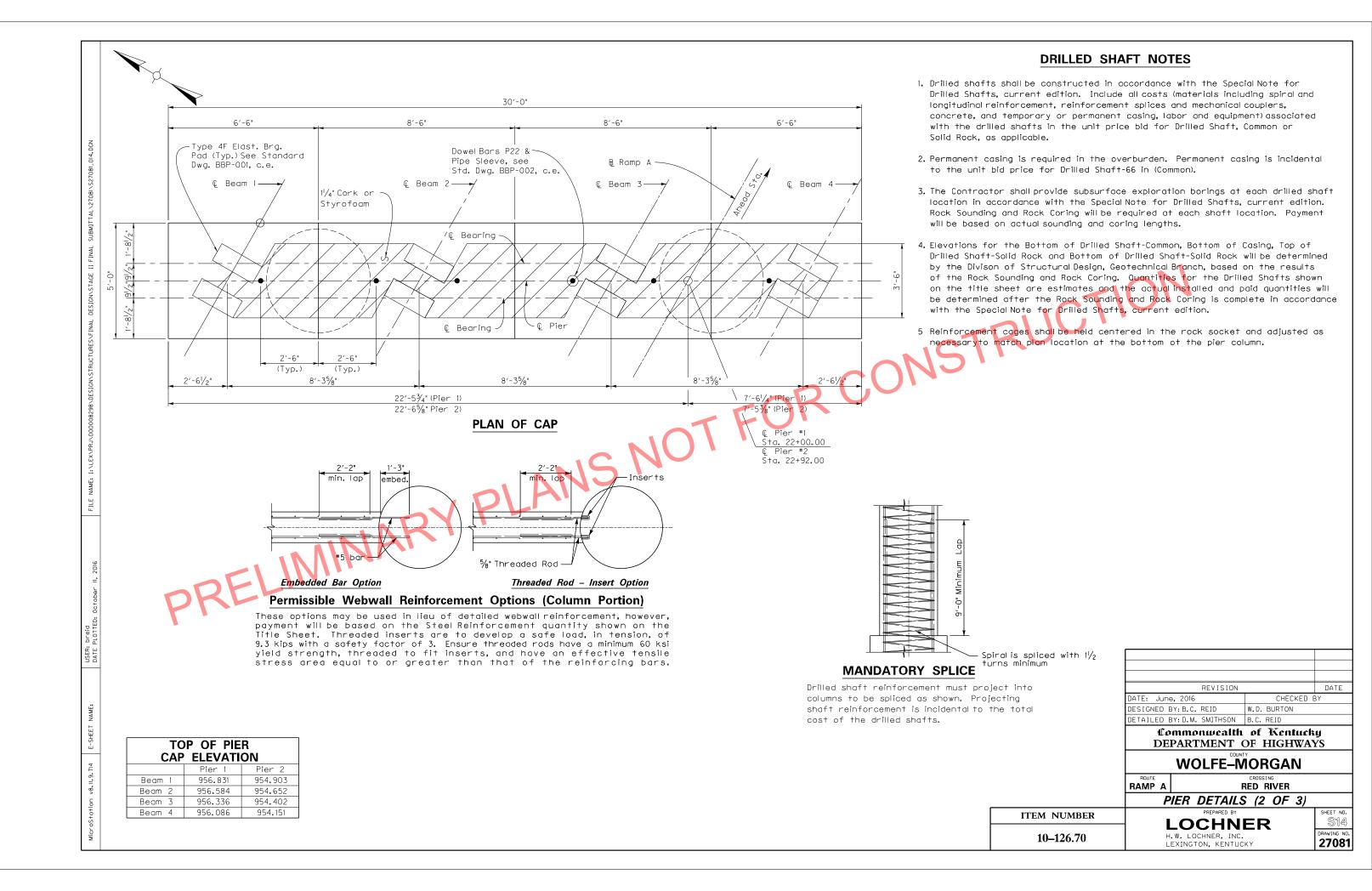
LOCHNER
H.W. LOCHNER, INC.
LEXINGTON, KENTUCKY

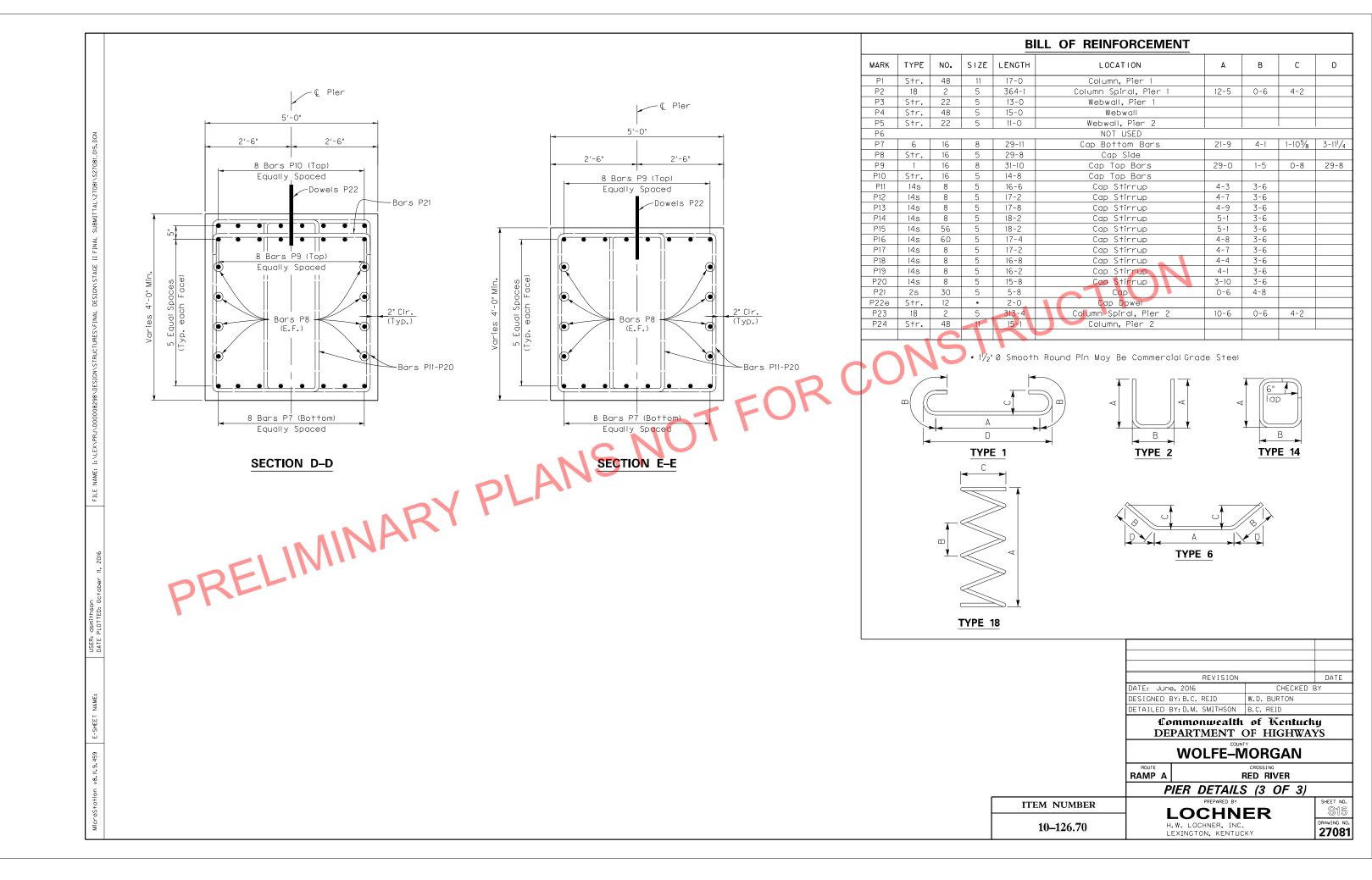
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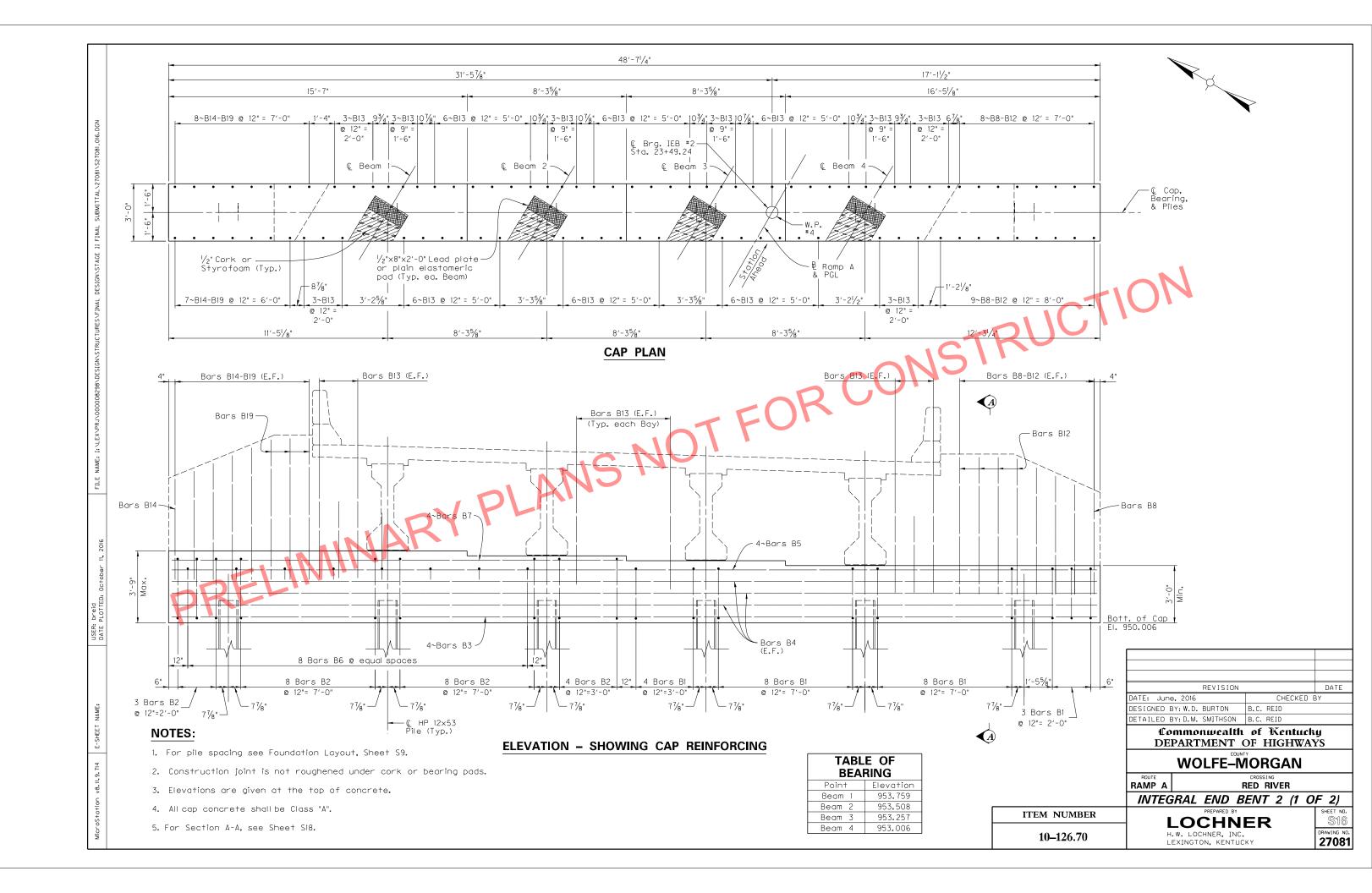


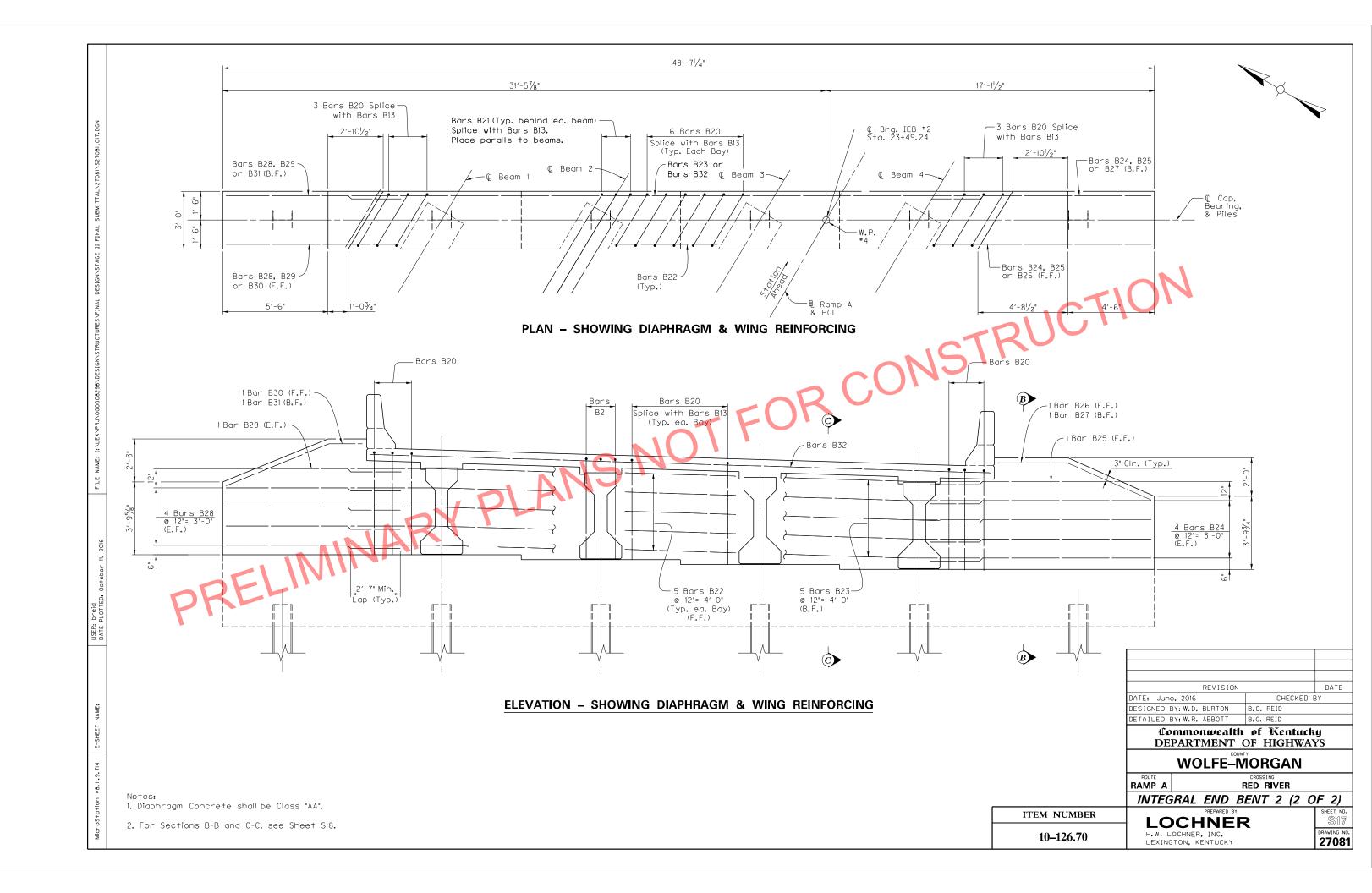


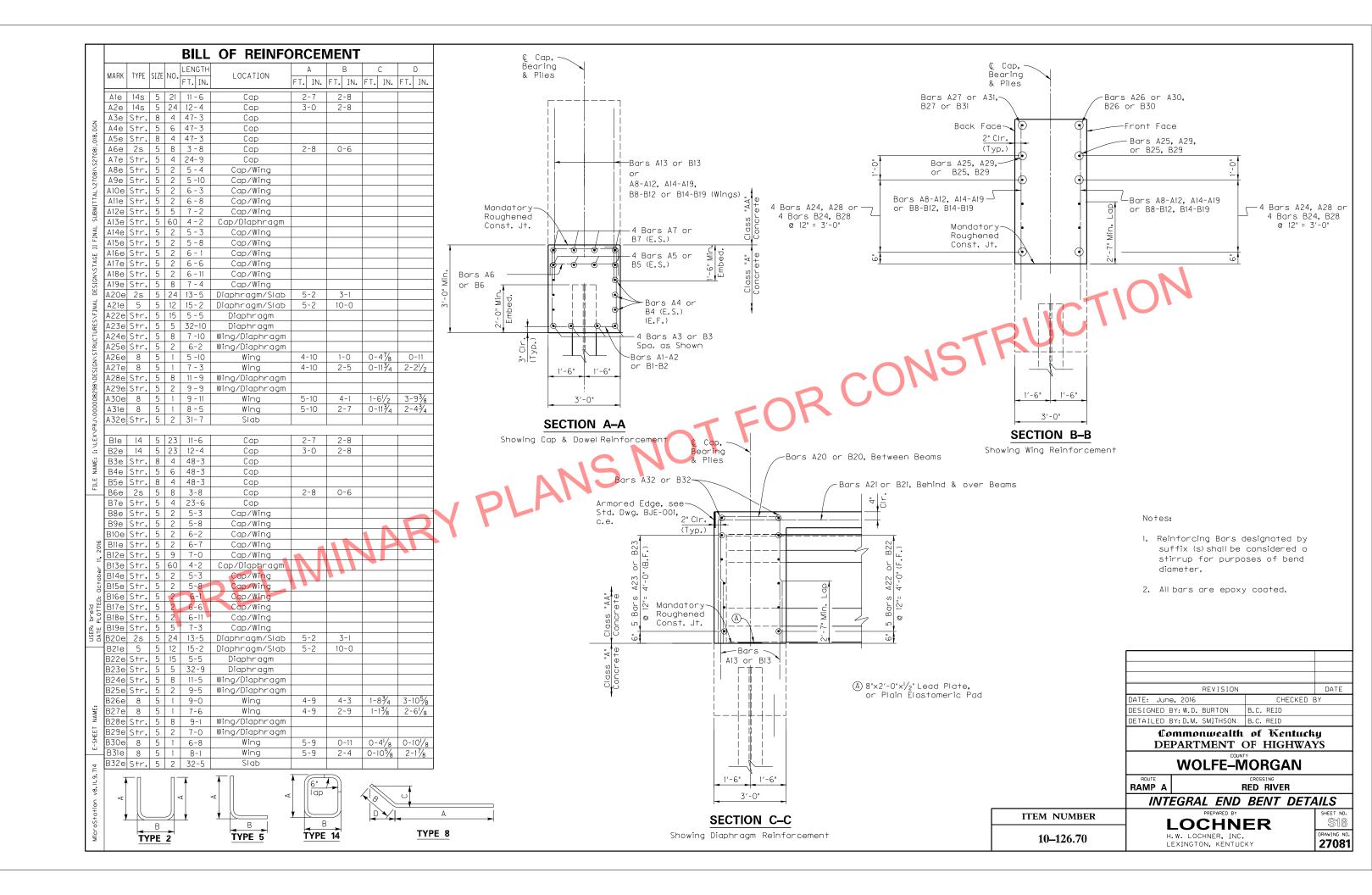


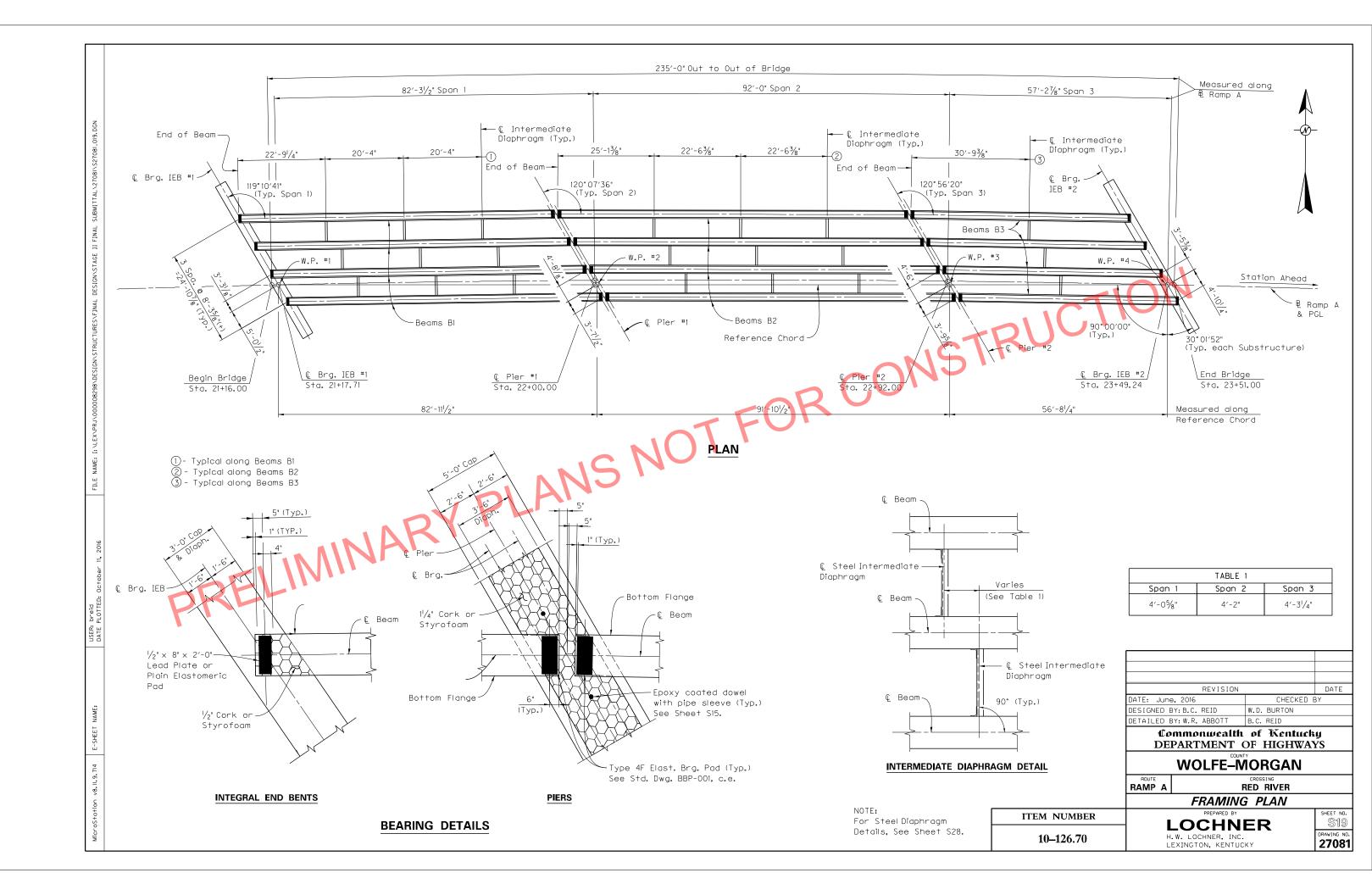


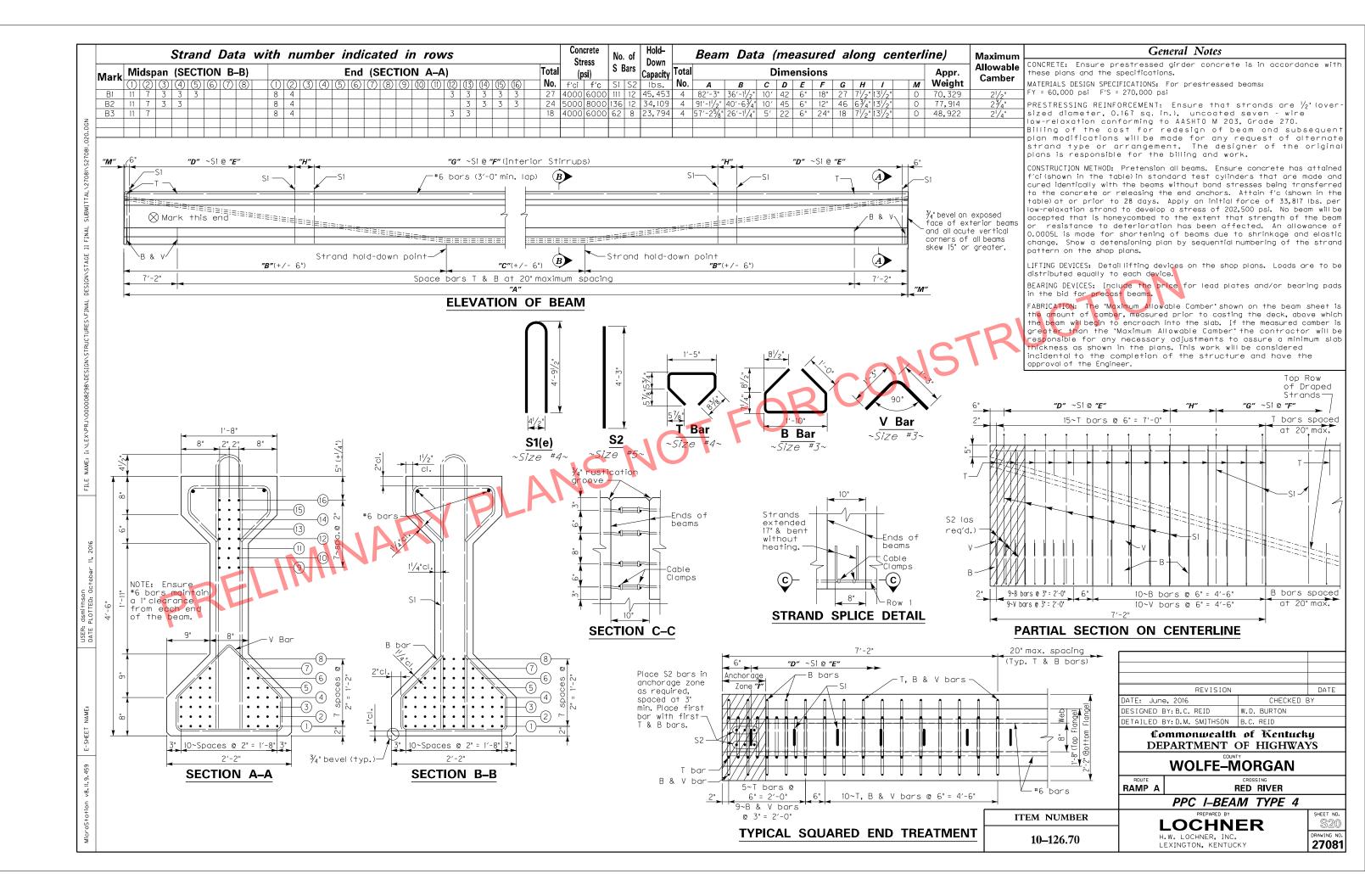


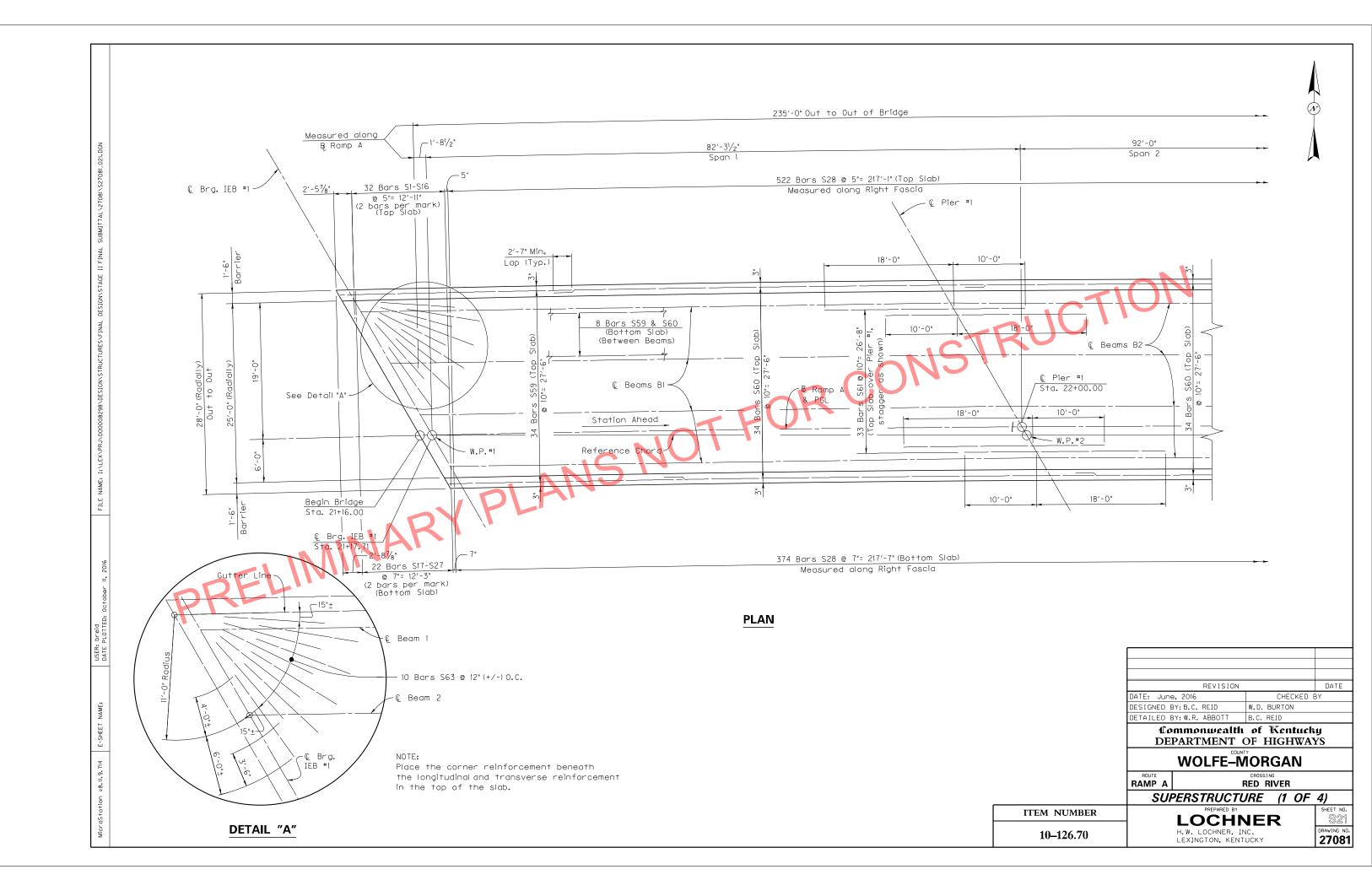


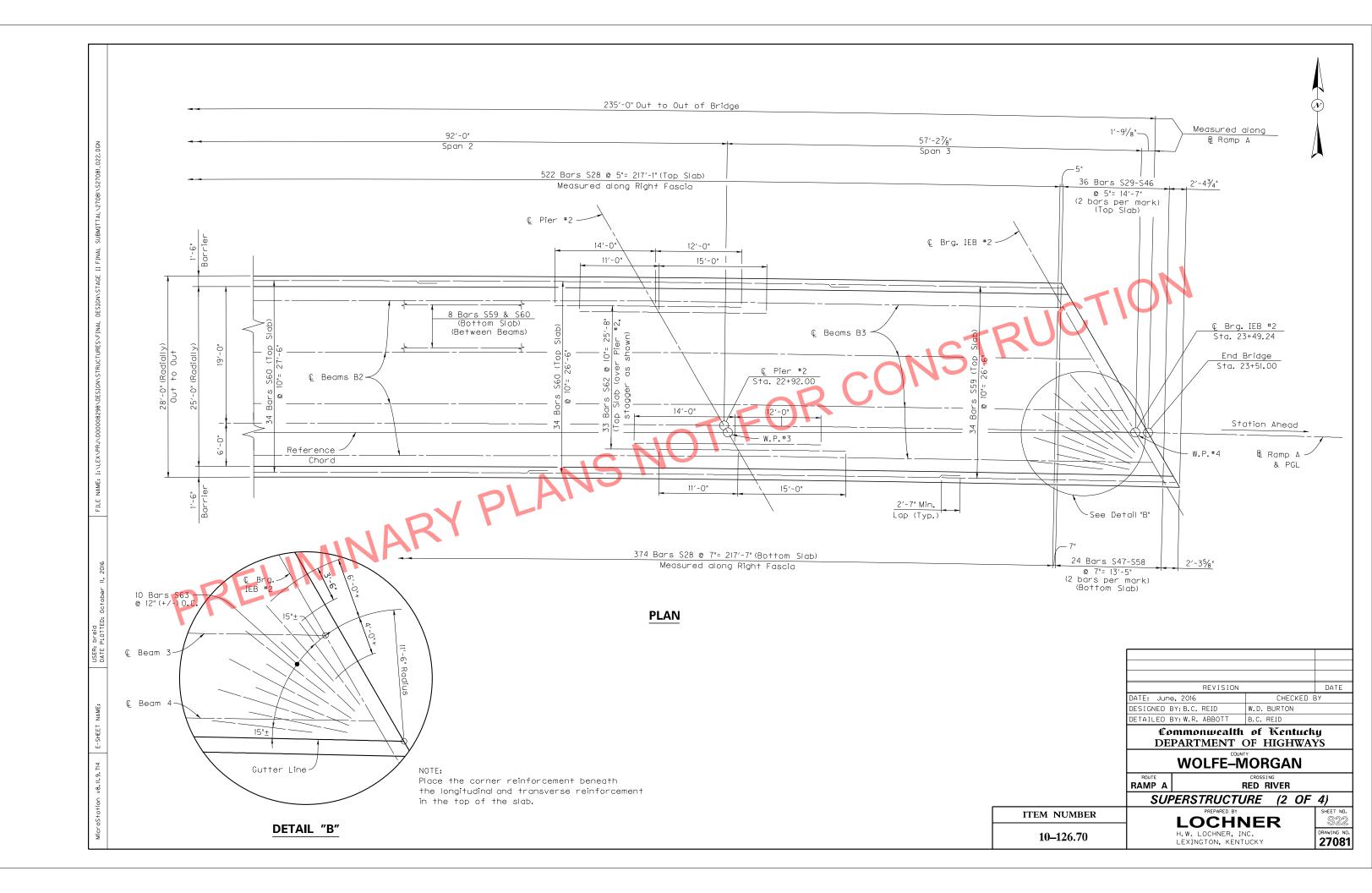


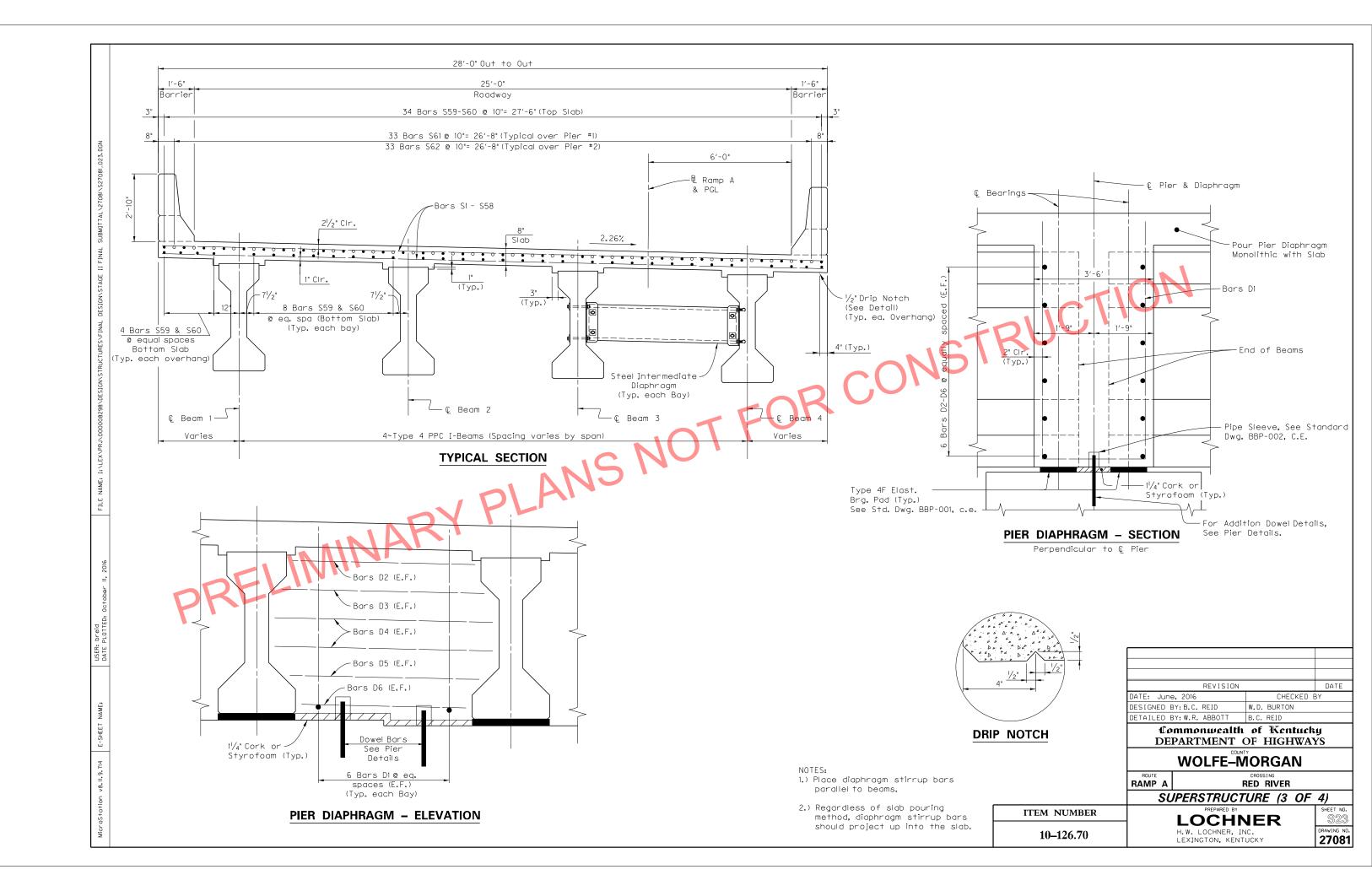












LENGTH MARK TYPESIZE NO. LOCATION FT. IN. Sle Str. 5 2 3 10 Top Slab S2e Str. 5 2 5 5 Top Slab S3e Str. 5 2 6 11 Top Slab

 S4e
 Str.
 5
 2
 8
 5

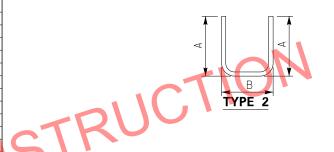
 S5e
 Str.
 5
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 10
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 Top Slab Top Slab S6e Str. 5 2 11 6 Top Slab S7e Str. 5 2 13 0 Top Slab S8e Str. 5 2 14 7 Top Slab S9e | Str. | 5 | 2 | 16 | Top Slab S10e Str. 5 2 17 7 Top Slab Sile Str. 5 2 19 2 Top Slab S12e Str. 5 2 20 8 Top Slab S13e Str. 5 | 2 | 22 | 2 Top Slab
 S14e
 Str.
 5
 2
 23
 8

 S15e
 Str.
 5
 2
 25
 2

 S16e
 Str.
 5
 2
 26
 8
 Top Slab Top Slab Top Slab S17e Str. 5 2 4 4 Bottom Slab Bottom Slab S18e Str. 5 2 6 6 6 S19e Str. 5 2 8 7 Bottom Slab S20e Str. 5 2 10 9 Bottom Slab S21e Str. 5 2 12 11 Bottom Slab S22e Str. 5 2 15 0 Bottom Slab S23e Str. 5 2 17 2 Bottom Slab S24e|Str.| 5 | 2 | 19 | 3 Bottom Slab |S25e|Str.| 5 | 2 | 21 | Bottom Slab S26e Str. 5 2 23 7 Bottom Slab S27e Str. 5 2 25 8 Bottom Slab Top & Bottom Slab S28e|Str.| 5 |896|27 | 8 | Top Slab S29e Str. 5 2 26 9 PRELIMINARY PLAI S30e Str. 5 2 25 5 S31e Str. 5 2 24 0 Top Slab S32e Str. 5 2 22 7 Top Slab \$33e Str. 5 2 21 2 Top Slab Top Slab Top Slab S36e Str. 5 2 17 0 Top Slab S37e Str. 5 2 15 8 Top Slab S38e Str. 5 2 14 3 Top Slab S39e Str. 5 2 12 10 Top Slab S40e Str. 5 | 2 | 11 | 5 Top Slab S41e Str. 5 2 10 1 Top Slab S42e Str. 5 2 8 8 Top Slab S43e Str. 5 2 7 3 Top Slab S44e Str. 5 2 5 Top Slab Top Slab S46e Str. 5 2 3 2 Top Slab S47e Str. 5 2 25 10 Bottom Slab S48e Str. 5 2 23 11 Bottom Slab S49e Str. 5 2 21 11 Bottom Slab Bottom Slab S51e Str. 5 2 18 1 Bottom Slab S52e Str. 5 2 16 Bottom Slab S53e Str. 5 | 2 | 14 | 2 Bottom Slab S54e Str. 5 | 2 | 12 | 3 Bottom Slab Bottom Slab Bottom Slab Bottom Slab S58e|Str.| 5 | 2 | 4 | 6 | Bottom Slab Top & Bottom Slab S59e Str. 5 | 132 | 32 | 6 Top & Bottom Slab S61e Str. 9 33 28 0 Over Pier #I S62e Str. 7 33 26 0 Over Pier #2 |S63e|Str.| 6 | 20 | 10 | 0 | Acute Corners

MARK	TVDE	CIZE		LENGTH		LOCATION		а		b		С		d
MARK	ITE	SIZE	NO.	FT.	IN.	LOCATION	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.
D1e	2s	5	36	13	0	Pier Diaphragm	4	-11	3	2				
D2e	Str.	5	12	5	1	Pier Diaphragm								
D3e	Str.	5	12	5	8	Pier Diaphragm								
D4e	Str.	5	24	6	1	Pier Diaphragm								
D5e	Str.	5	12	5	3	Pier Diaphragm								
D6e	Str.	5	12	4	7	Pier Diaphragm								



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FT. IN. FT. IN. FT. IN. FT. IN.

С

NOTE:

All Reinforcing Bars are Epoxy Coated.

Commonwealth	of Kentuck	u
DETAILED BY: W.R. ABBOTT	B.C. REID	
DESIGNED BY: B.C. REID	W.D. BURTON	
DATE: June, 2016	CHECKED E	3 Y
REVISION		DATE

DEPARTMENT OF HIGHWAYS

WOLFE-MORGAN

RAMP A RED RIVER

SUPERSTRUCTURE (4 OF 4)

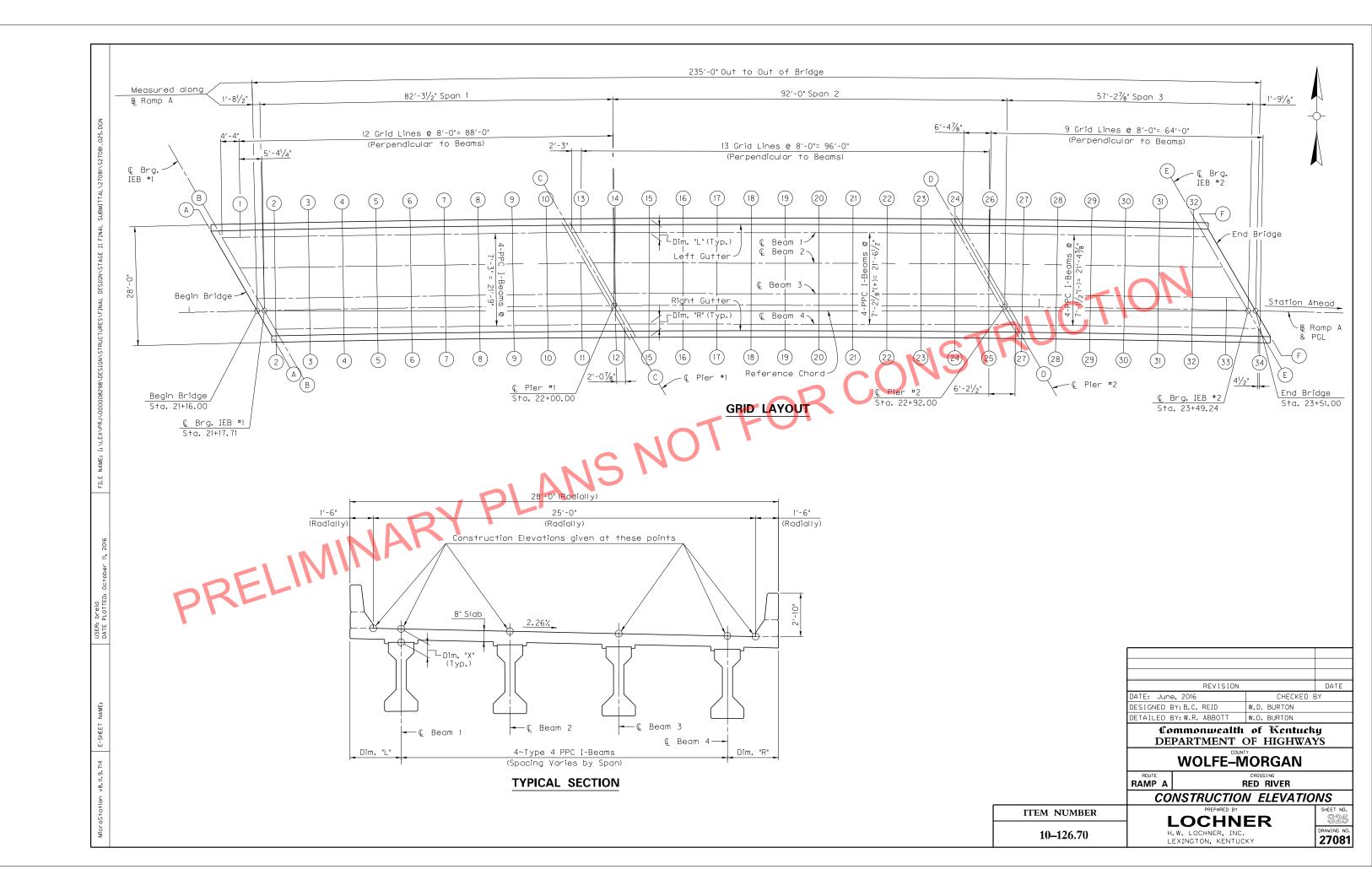
ITEM NUMBER

10-126.70

LOCHNER

H. W. LOCHNER, INC.
LEXINGTON, KENTUCKY

SHEET NO. \$24 DRAWING NO. 27081



	Dir. III	1 - C+		î Beam 1			î Beam 2			© Beam 3		Q Beam 4			Disk	D: D
LOCATION	Dim. "L" (F†.)	Left Gutter	Const. Elev.	Top of Beam	Dim. "X"	Const. Elev.	Top of Beam	Dim. "X"	Const. Elev.	Top of Beam	Dim. "X"	Const. Elev.	Top of Beam	Dim. "X"	Right Gutter	Dim. "R' (F †.)
Skew Line AA		963.766	963.719			963.499			963.277			963.054			963.005	
Skew Line BB			963.695			963,474			963.251			963.028			962.979	
Skew Line CC			962.285			962.038			961, 789			961,540			961.482	
Skew Line DD			960.356			960.106			959.856			959.605			959.542	
Skew Line EE		959.219				958,903			958,652			958.401			958.335	
Skew Line FF		959.182				958.867			958.616			958.364			958.298	
Grid Line I	3,112		963,643													
Grid Line 2	3.170	963.580				963.369			963.198							3.080
Grid Line 3	3.217	963.473				963,262			963.092			962.920			962.883	3.033
Grid Line 4	3.252	963.356				963.147			962.978			962.808			962.773	2.998
Grid Line 5	3.275	963.230				963.024			962.856			962.688			962.654	2.976
Grid Line 6	3.285	963.093				962.890			962.725			962.560			962.526	2.965
Grid Line 7	3.283	962.947				962.747			962.584			962.421			962.388	2.967
Grid Line 8	3.269		962.752			962.594			962.433			962.272			962.239	2.981
Grid Line 9	3.243	962.626				962.430			962.273			962.114			962.081	3.007
Grid Line 10	3.205	962.453				962.260			962.103			961.946			961.912	3.045
Grid Line II						962.083			961.927			961.770			961.736	3.096
Grid Line 12												961.588			961.551	3.158
Grid Line 13	3,191	962.285	962 244													
Grid Line 14	3.261	962.143				961.928			961.754							
Grid Line 15	3.318	961.996				961. 782			961.608			961.436			961.397	3.136
Grid Line 16	3.364	961.846				961.632			961.460			961.289			961.251	3.090
Grid Line 17	3.397	961.690				961.479			961.309			961.139			961.102	3.057
Grid Line 18	3.418	961.527				961.320			961.153			960.986			960, 951	3,036
Grid Line 19	3.428	961.359				961.155			960.990			960.827			960.792	3.026
Grid Line 20	3.424	961.184				960.983			960.822			960.662	\rightarrow		960.627	3.030
Grid Line 21	3.409	961.003				960.805			960.647			960.490			960.456	3.045
Grid Line 22	3.382		960.775			960.622			960.466			960.312			960.278	3.072
Grid Line 23	3.343	960.626				960.434			960.281			960.128			960.094	3.112
Grid Line 24	3.291		960.396			960,244			960.092			959.941			959.906	3.164
Grid Line 25	J. Z J1								959.901) 		959.750			959.714	3.228
Grid Line 26	3.317		960.226			960.063									333,114	J. Z.C.
Grid Line 27	3.354		960.062			959.900			959.737			959.573			959.532	3.279
	3.379	959.939				959.735			959.573			959.410			959.369	3.254
Grid Lino 28						959.569	 		959.407			959.245			959.205	3.241
Grid Line 28		959 772							222.701						959.039	
Grid Line 29	3.392	959.772				959 400			959 240			1 959 079 1				3 240
Grid Line 29 Grid Line 30	3.392 3.393	959.602	959.560		21	959.400			959.240			959.079				3.240
Grid Line 29 Grid Line 30 Grid Line 31	3.392 3.393 3.382	959.602 959.430	959.560 959.388		21	959.229			959.070			958.909			958.870	3.251
Grid Line 29 Grid Line 30	3.392 3.393 3.382 3.359	959.602 959.430 959.256	959.560 959.388		21											

NOTES FOR ELEVATIONS TAKEN ON PRESTRESSED CONCRETE BEAMS

Take elevations on top of beam at points indicated by the grid layout. The beam elevations are to be read to three decimals and entered into table under "Top of Beam" elevations.

Compute Dimension "X" as follows: "Construction Elevation" minus "Top of Beam" elevation equals Dimension "X". Construction elevations include camber due to weight of concrete slab and barrier. Measuring of Dimension "X" gives the final check on beam tolerances for camber, beam damage and errors in erection that produce reverse combers, sags and unsightly fascia beams.

For setting templates, measure Dimension "X" above top of beam for top of template. Do NOT set template by elevations.

Temporary supports or shoring will not be permitted under the girders when pouring the concrete floor or slab or when taking the "Top of Beam" elevations.

Construct barriers to roadway grade. Do NOT add camber to the barrier.

Note to Engineer: The "Maximum Allowable Camber" shown on the beam sheet is the amount of camber, measured prior to casting the deck, above which the beam will begin to encroach into the slab. If the measured camber is greater than the "Maximum Allowable Camber" the Contractor will be responsible for any necessary adjustments to assure a minimum slab thickness of 8 inches as shown in the plans. This work will be considered incidental to the completion of the structure and must have the approval of the Engineer.

REVISION		DATE
DATE: June, 2016	CHECKED E	3 Y
DESIGNED BY: B.C. REID	W.D. BURTON	
DETAILED BY: W.R. ABBOTT	B.C. REID	
Commonwealth	of Kentuck	y

DEPARTMENT OF HIGHWAYS

WOLFE-MORGAN

RAMP A

RED RIVER

CONSTRUCTION ELEVATIONS

ITEM NUMBER

10-126.70

LOCHNER

H. W. LOCHNER, INC.
LEXINGTON, KENTUCKY



